

# The fundamental diagram of traffic flow and its role in modeling traffic dynamics

A Seminar Presentation at  
**Traffic Modeling and Management: Trends and Perspectives**  
at [INRIA Sophia Antipolis – Méditerranée](#)

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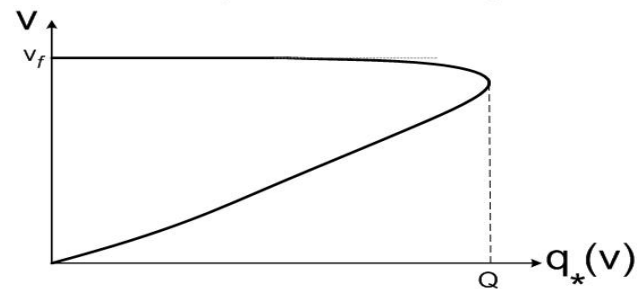
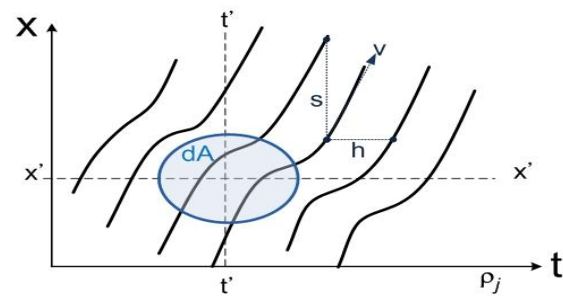
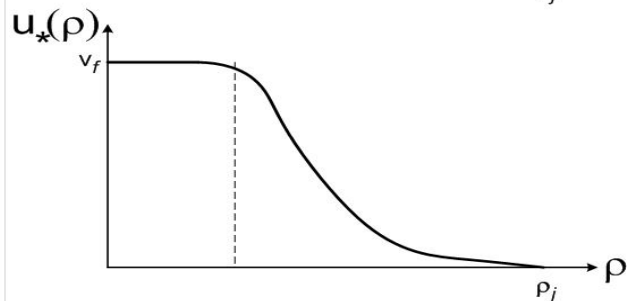
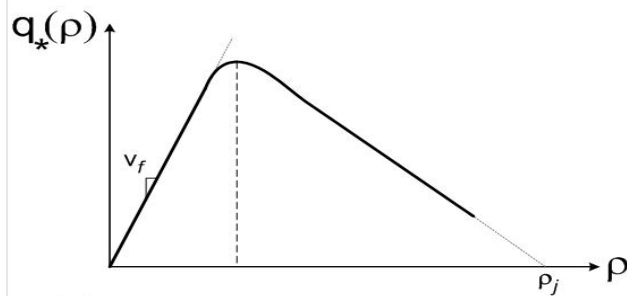
March 22, 2013

# Outline of Presentation

- Concepts of the fundamental diagram (FD) and its role in mathematical traffic flow theory
- Overview of various forms of the FD and their empirical evidence
- The use of boundary conditions for screening FD forms
- The FD's influence on traffic dynamics
- Summary and outlook

# The fundamental diagram of traffic flow

- First coined by Haight (1963), it refers to the flow-density curve either observed empirically or obtained from car-following models at that time
- In this talk we use the term to refer *any* of the pair-wise relations between flow (headway), density (spacing), and speed either at a fixed location, or for a moving platoon of vehicles



# The Fundamental Diagram (FD)

- Embodies driver behavior that separates traffic flow from other material fluids
- Forms the foundation of some transportation applications (e.g., highway capacity and level of service analysis)
- Permeates in all levels of mathematical description of traffic flow
  - In microscopic, they are linked to steady-state behavior of car-following or CA models, or enter these models a priori
  - In macroscopic or mesoscopic, it enters into the relaxation process of the acceleration or “momentum” equation

# The role of the FD in traffic models

- Microscopic

- Modified Pipes' model
- Newell' Model
- Bando' model

$$\dot{x}_n = \min \{ v_f, (s_n(t) - l) / \tau \}$$

$$\dot{x}_n(t + \tau) = v_f \left[ 1 - \exp \left\{ -\lambda (s_n(t) - l) / v_f \right\} \right]$$

$$\ddot{x}_n(t) = a \left[ (u_*(s_n) - \dot{x}_n)(t) \right], a = 1 / \tau$$

$$\rho = 1 / s, u_*(s) = v_*(\rho), q = \rho v, q_*(\rho) = \rho v_*(\rho)$$

- Macroscopic continuum

- LWR model
- Payne-Whitham model
- Aw-Rascle, Zhang model

$$\rho_t + q_*(\rho)_x = 0$$

$$\rho_t + (\rho v)_x = 0, \quad v_t + (v v)_x + \frac{c_0^2}{\rho} \rho_x = \frac{v_*(\rho) - v}{\tau}$$

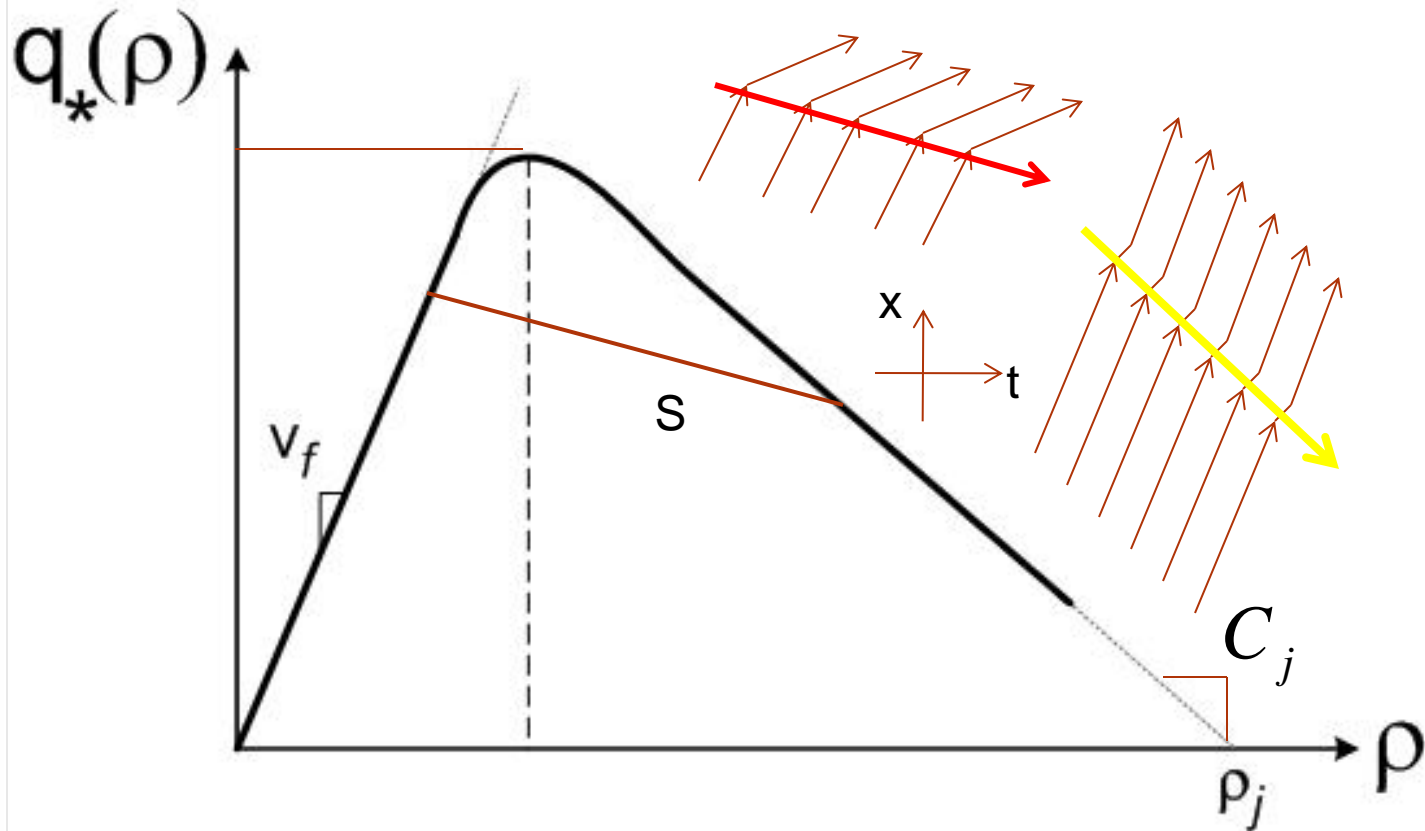
$$\rho_t + (\rho v)_x = 0, \quad v_t + (v - c(\rho)) v_x = \frac{v_*(\rho) - v}{\tau}$$

$$c(\rho) = -\rho v_*'(\rho)$$

# FD and traffic waves

$$\rho_t + q_*(\rho)_x = 0$$

$$\frac{dx}{dt} = \frac{dq_*}{d\rho}, \quad \frac{d\rho}{dt} = 0$$

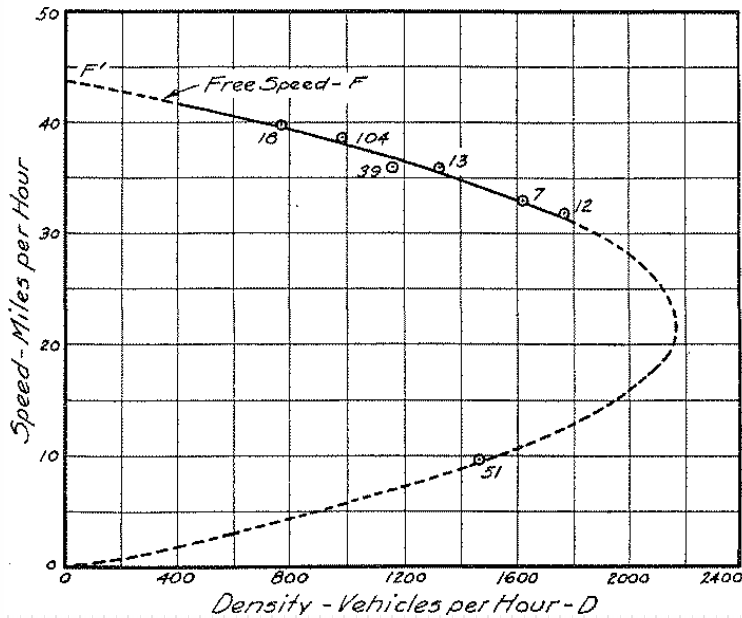


# The FD comes in many forms

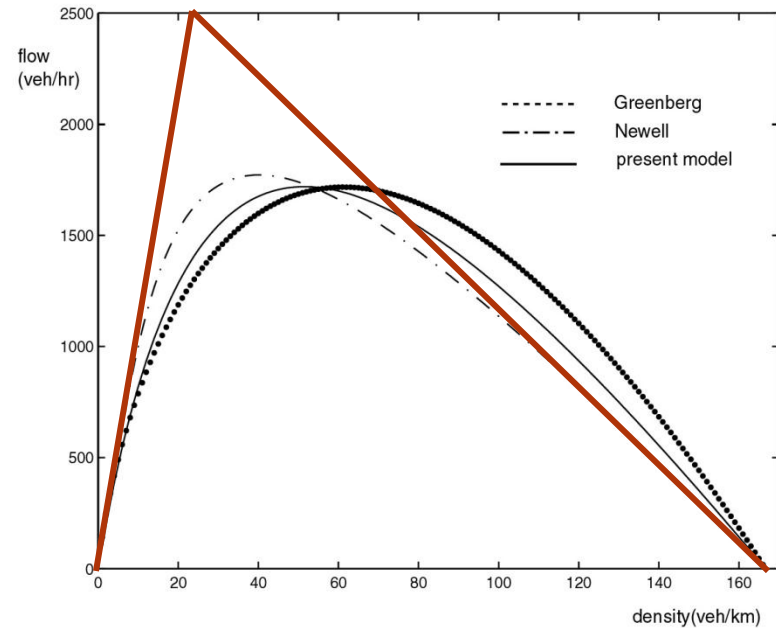
- Continuous/smooth and concave
- Continuous/smooth and concave-convex
- Discontinuous, piece-wise smooth (possibly multi-valued)
- Multi-phase, set-valued

# Continuous/smooth, concave FD

$$u_*(s) = v_f \left[ 1 - \exp \left\{ -\lambda(s-l) / v_f \right\} \right]$$



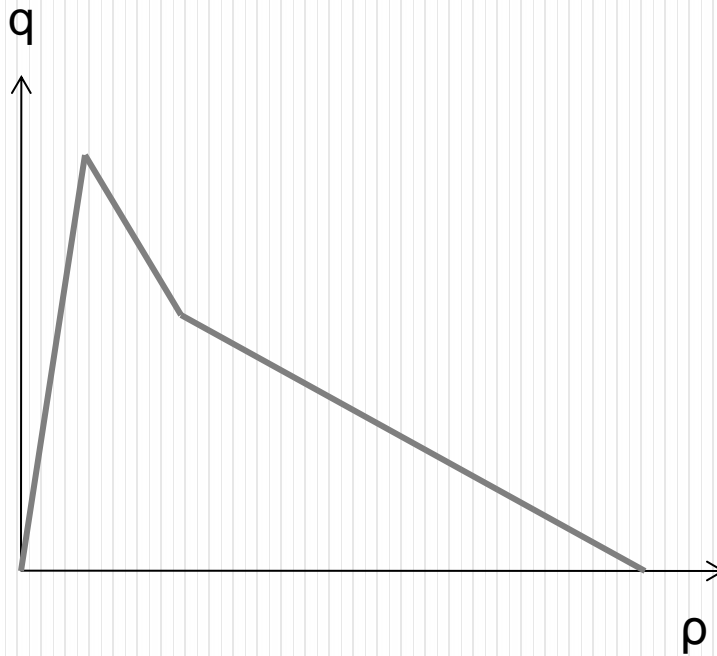
Greenshields (1935)



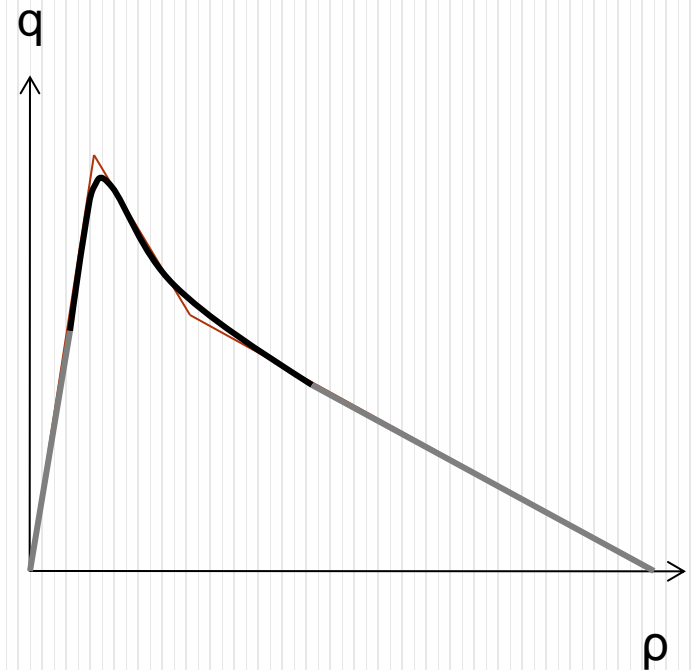
Other common concave FDs



# Continuous/smooth, concave-convex

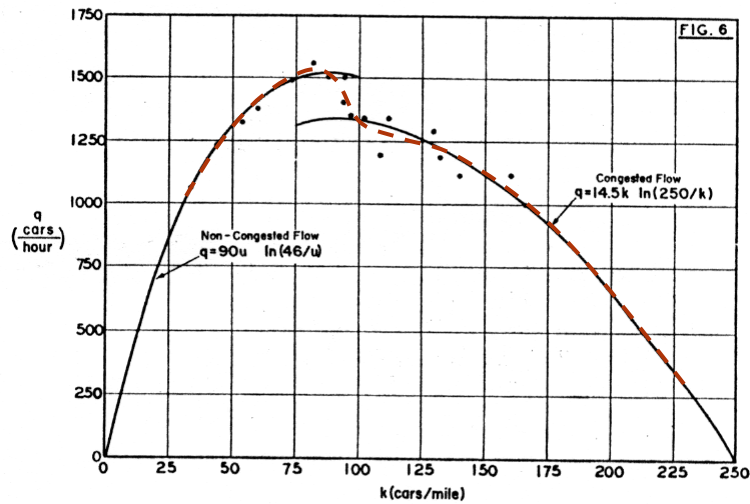


piece-wise linear, concave-convex FD

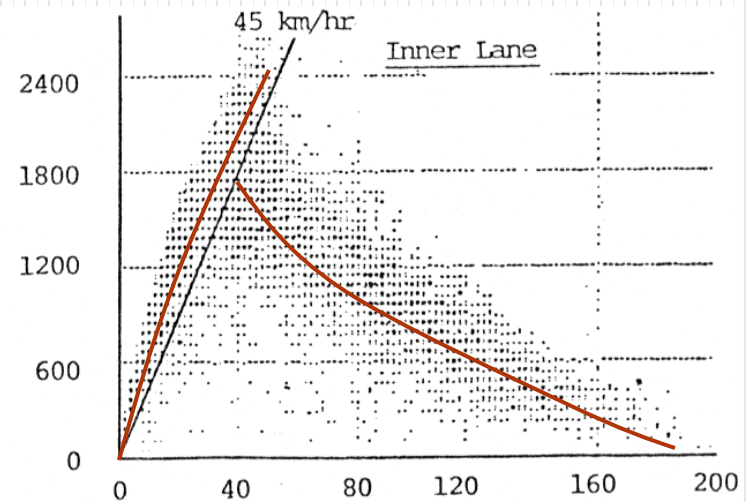


smooth, concave-convex FD  
(e.g. Bando 1995)

# Discontinuous, piece-wise smooth (possibly multi-valued)

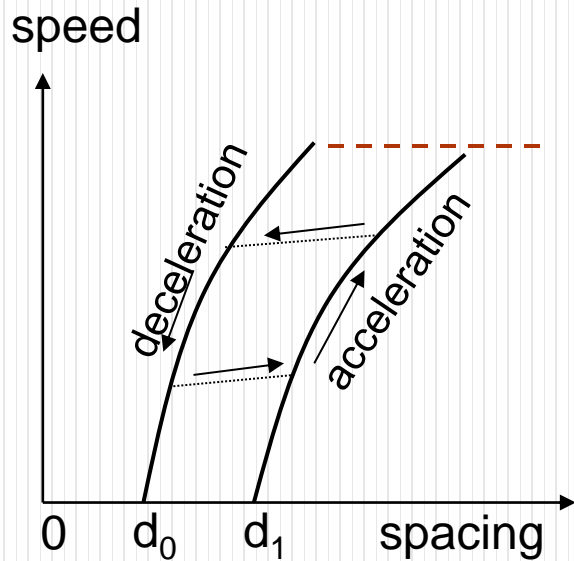


Eddie (1961)

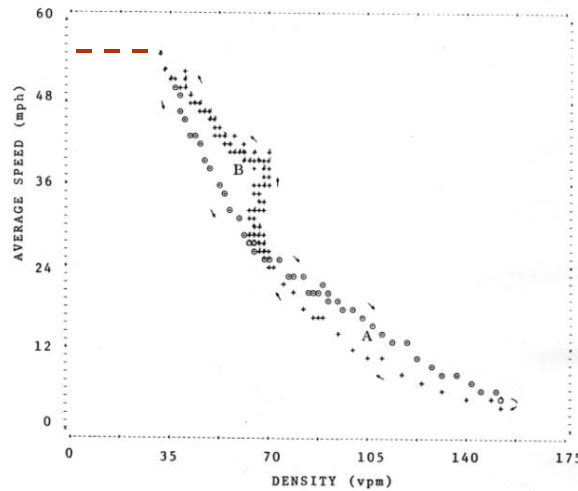


Koshi et al. (1983)

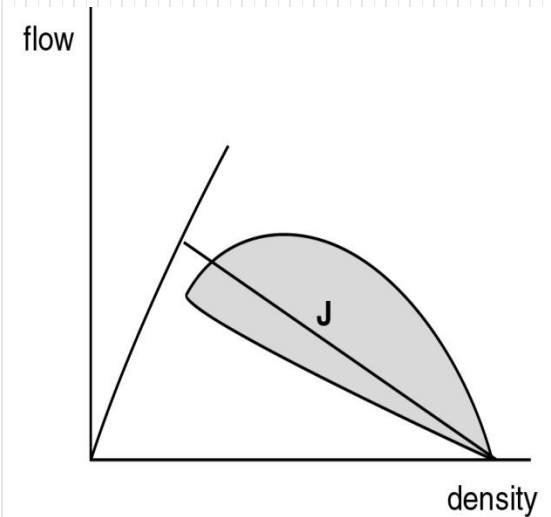
# Multi-phase, set-valued



Newell(1962)



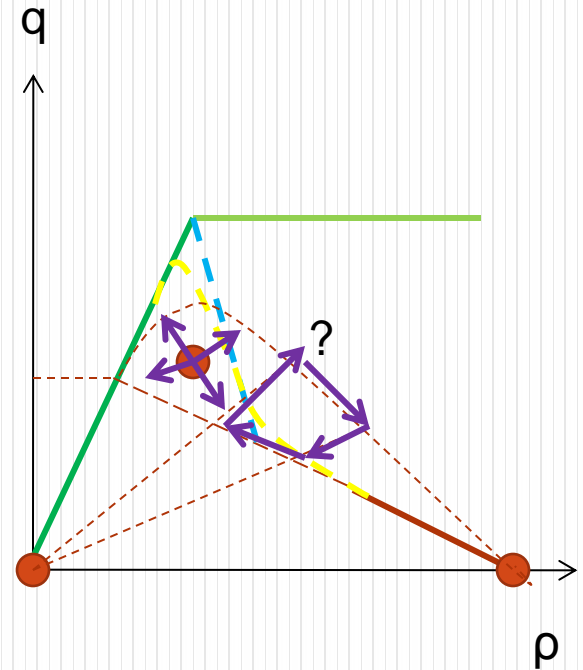
Treiterer and Myers(1974)



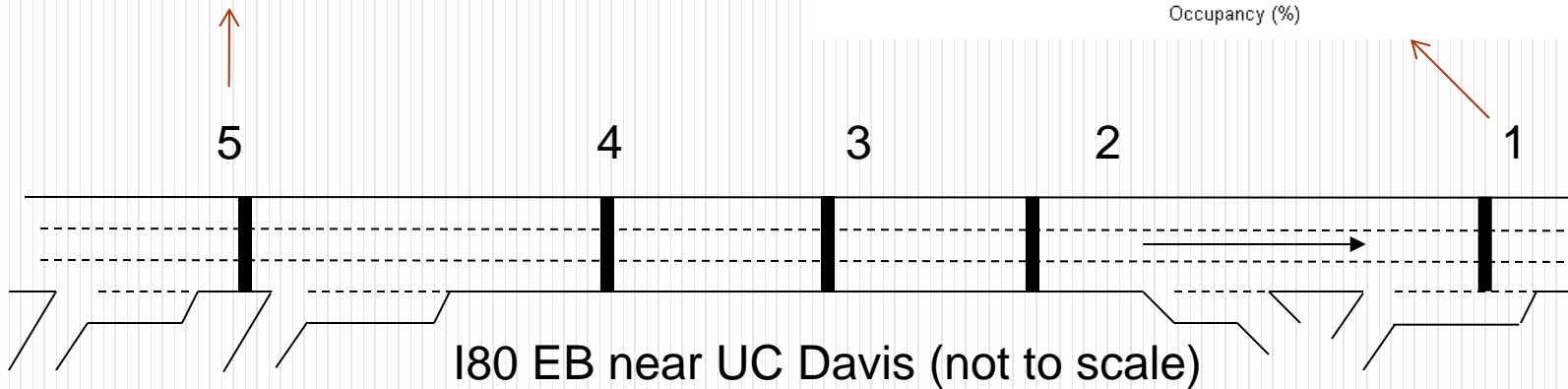
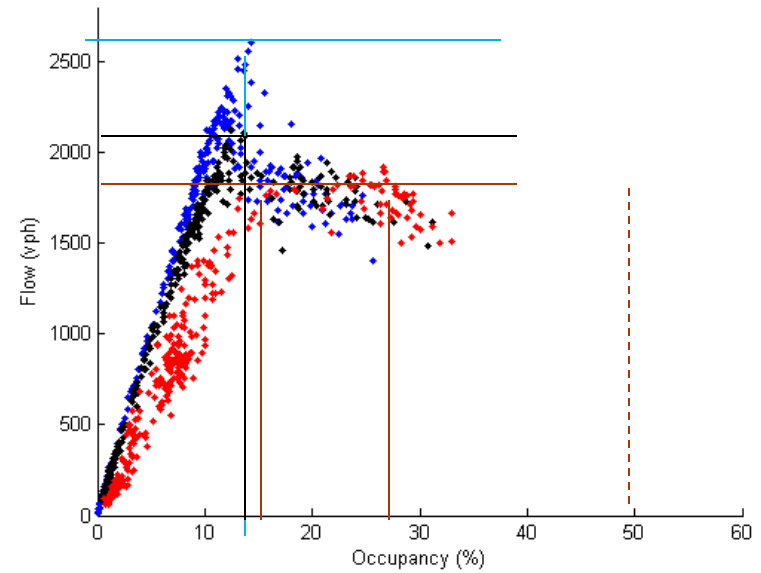
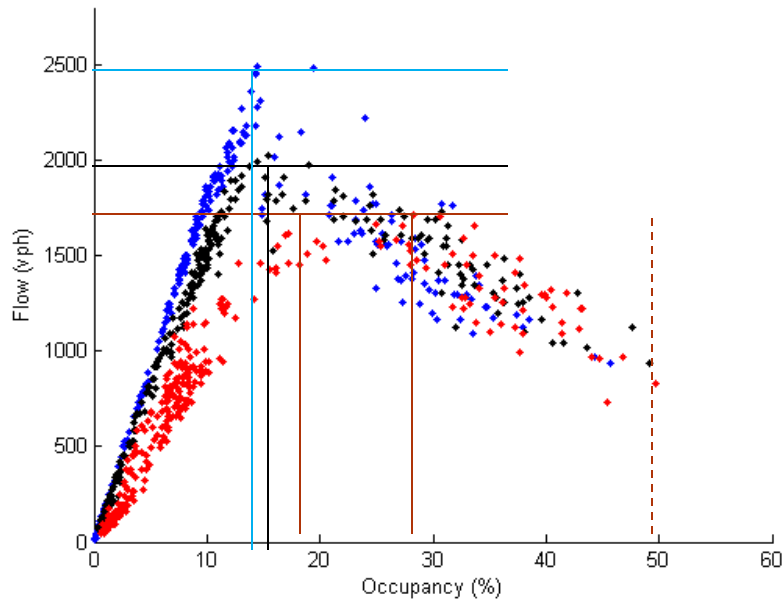
Kerner (1998),  
reproduced

# Essential properties of FD

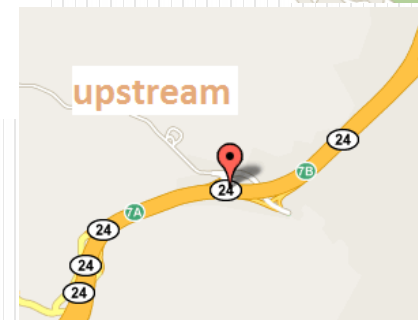
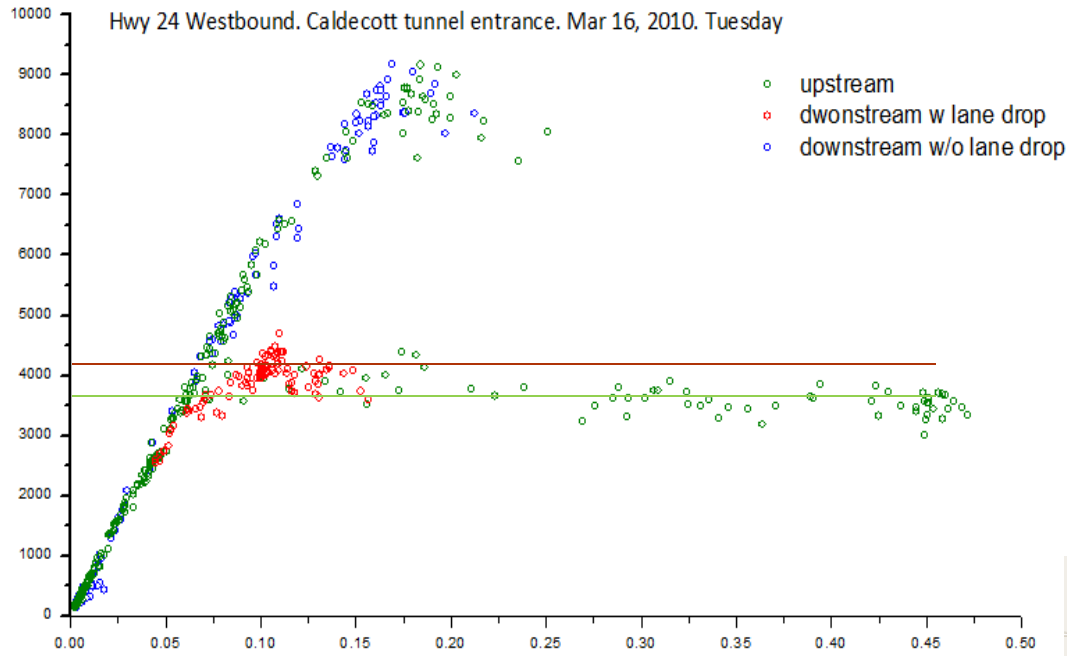
- No vehicle, no flow
- Physical limit on speed ( $v_f$ )
- Physical storage limit (jam density)
- Jam packed, no velocity
- Physical bound on flow
  - $v_f / \text{min safe spacing} (=v_f \cdot \text{reaction time} + \text{veh length})$
- Jam dissipation
  - Wave speed
    - Vehicle length / reaction time<sup>2</sup>
    - (-) 6 meter / 1 sec = 21.6 km/hr = 13.4 mph
    - (-) 6 meter / 1.8 sec = 12 km/hr = 7.5 mph
  - Flow discharge rate
    - 2 seconds headway  $\Rightarrow$  1800 vph



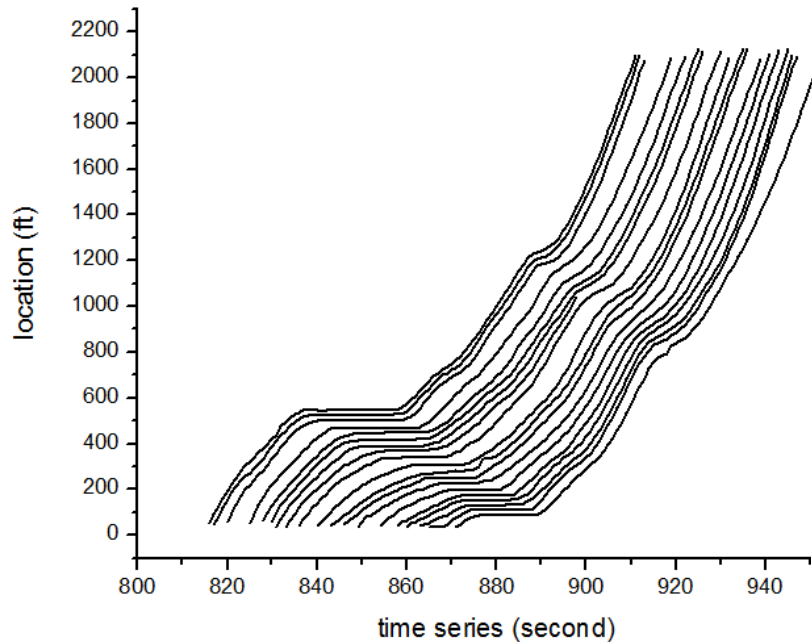
# Empirical evidence: fixed locations



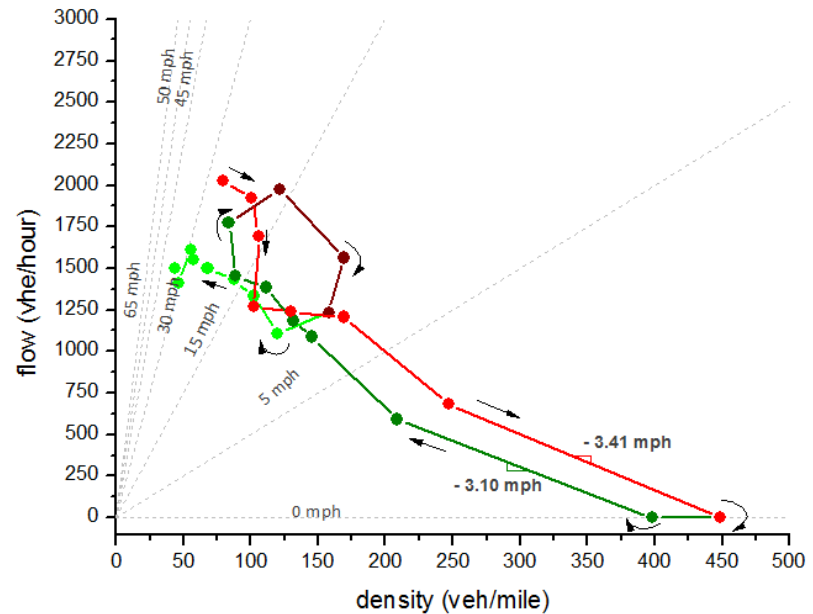
# Empirical evidence: bottleneck effects



# Empirical evidence: moving platoons (1)

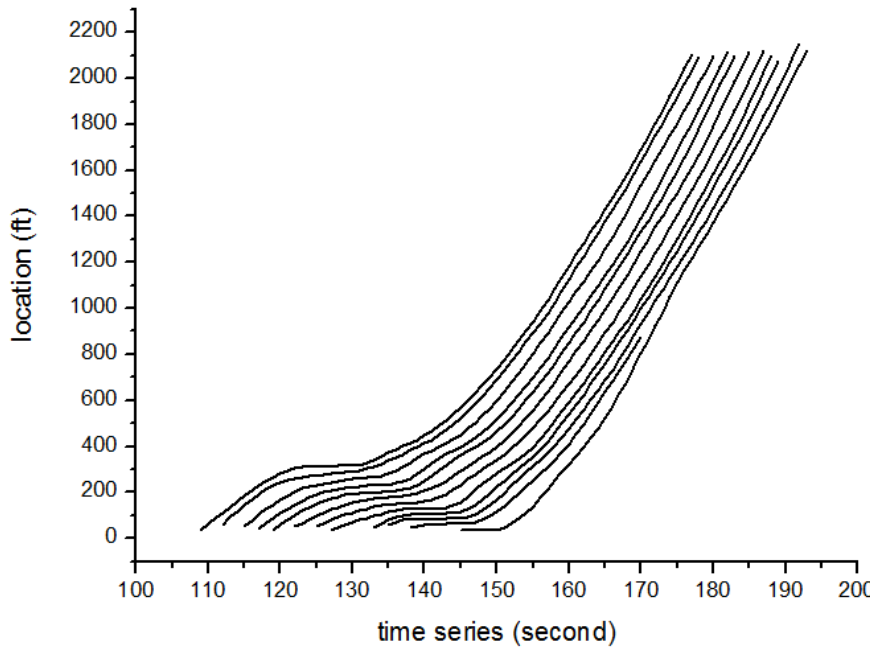


Vehicle platoon traveling through two shock waves

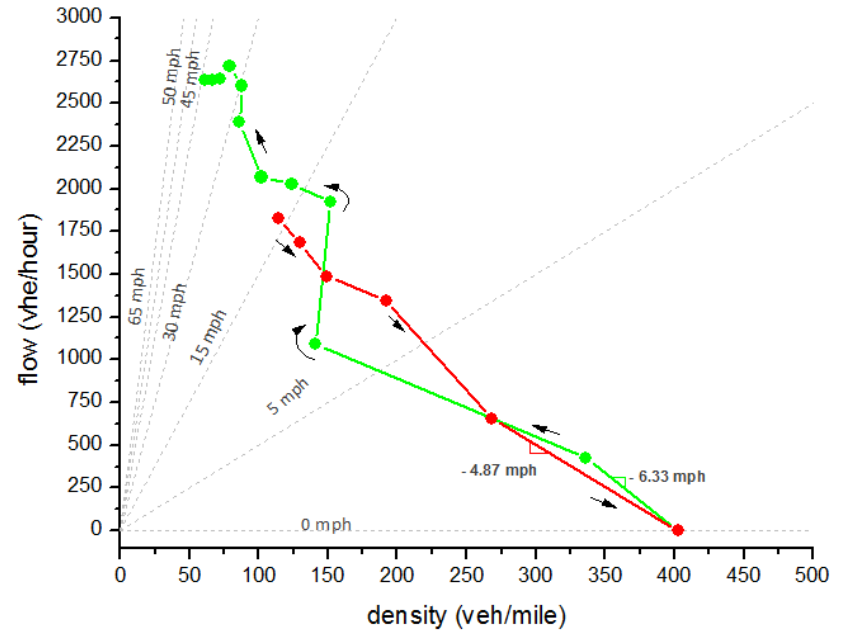


flow-density phase plot

# Empirical evidence: moving platoons (2)



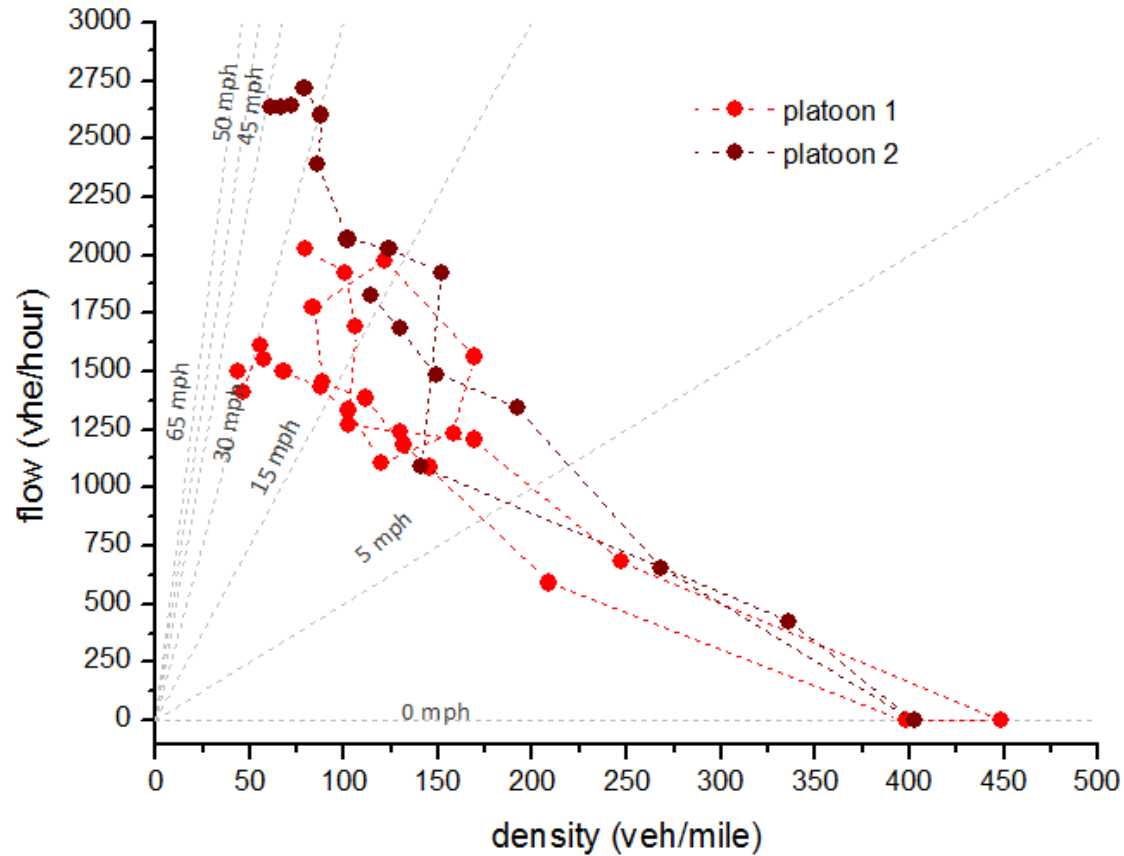
Vehicle platoon traveling through one shock wave



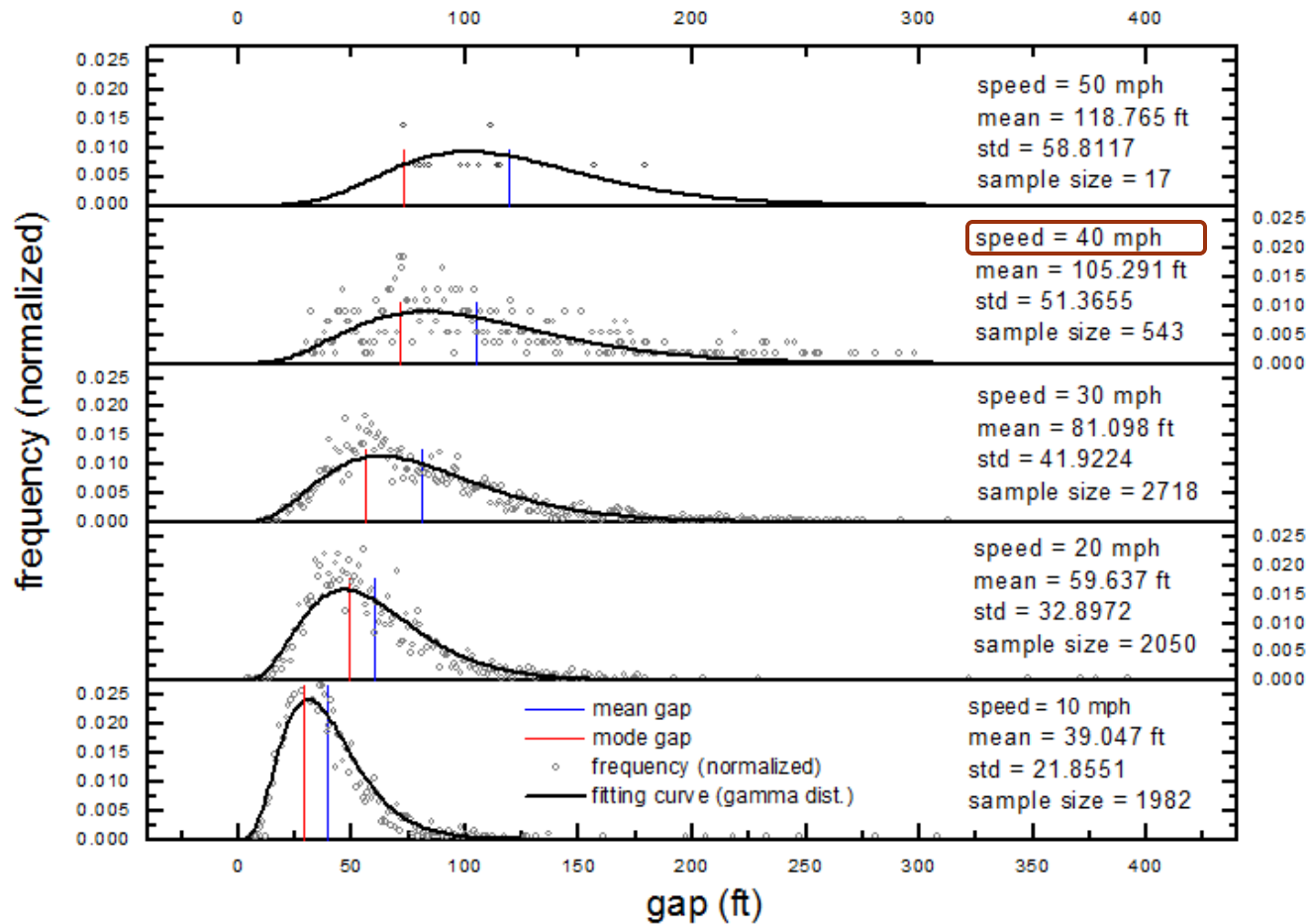
flow-density phase plot



# Phase plots of both platoons

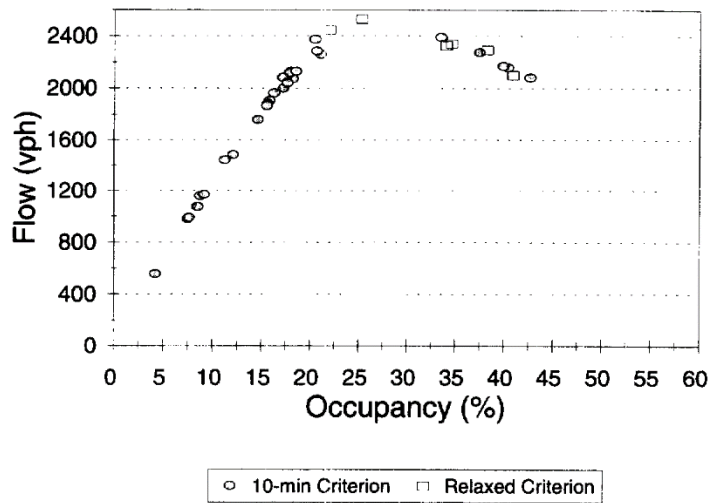


# Empirical evidence: stochasticity in a large ensemble

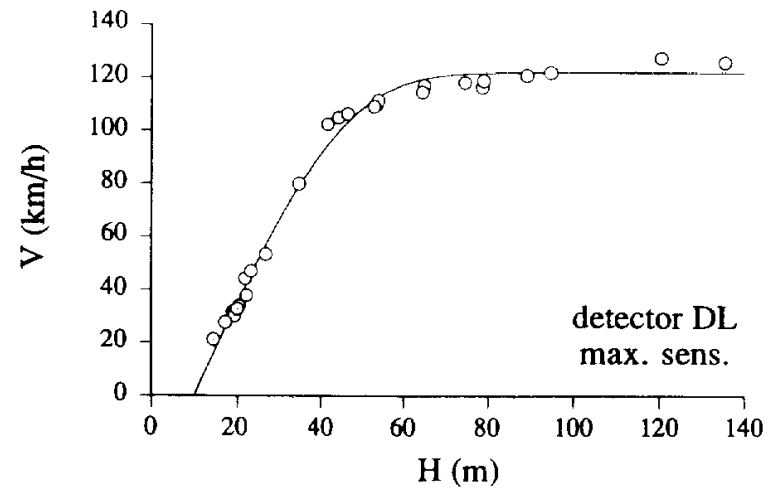


# Empirical evidence: stationary flow

$$V = V_f \left\{ 1 - \exp \left[ 1 - \exp \left( \frac{|C_f|}{V_f} \left( \frac{H}{H_f} - 1 \right) \right) \right] \right\}$$



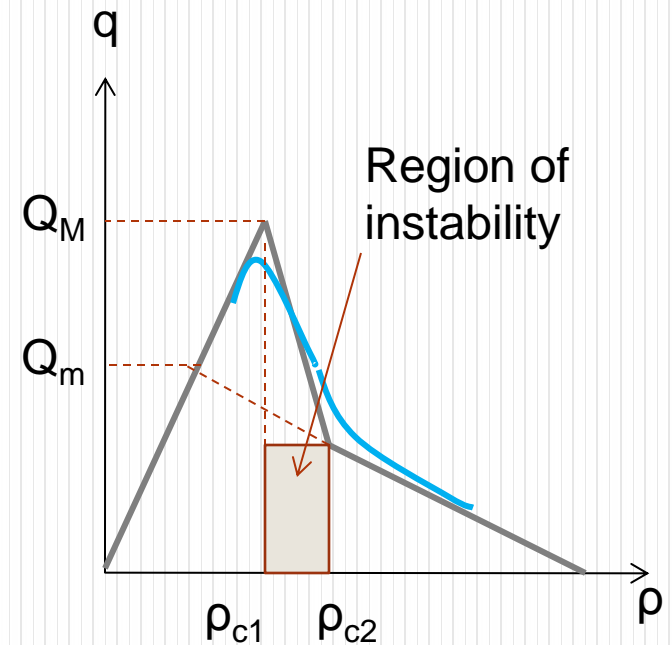
Cassidy (1998) [QEW data]



Del Castillo (1995) [Freeway A2 Amsterdam data]

# Which form after all?

- The form we choose must
  - Respect the boundary conditions at zero and jam densities
  - Have physically meaningful and calibratable parameters
  - Produce reasonable capacity
  - Capture the flow drop at free-flow to congestion transition
  - Be simple, yet produce the essential dynamic features of traffic flow, such as accel/decel asymmetry, when being incorporated in traffic models
- The answer: single-valued concave-convex FD



# Linear stability analysis

Newell

$$\dot{x}_n(t + \tau) = v_f \left[ 1 - \exp \left\{ -\lambda (s_n(t) - l) / v_f \right\} \right]$$

Bando

$$\ddot{x}_n(t) = a \left[ \left( u_*(s_n) - \dot{x}_n \right) (t) \right], \quad a = 1 / \tau$$

Payne-Whitham

$$\rho_t + (\rho v)_x = 0, \quad v_t + (v v)_x + \frac{c_0^2}{\rho} \rho_x = \frac{v_*(\rho) - v}{\tau}$$

Obtained by perturbing a steady-state solution and study the growth/decay of the perturbation over vehicle numbers (micro) or time-space (macro) using Fourier transforms

# Instability and its relation to the shape of the fundamental diagram

$$-\bar{\rho}v'_*(\bar{\rho}) > c_0$$

(PW)

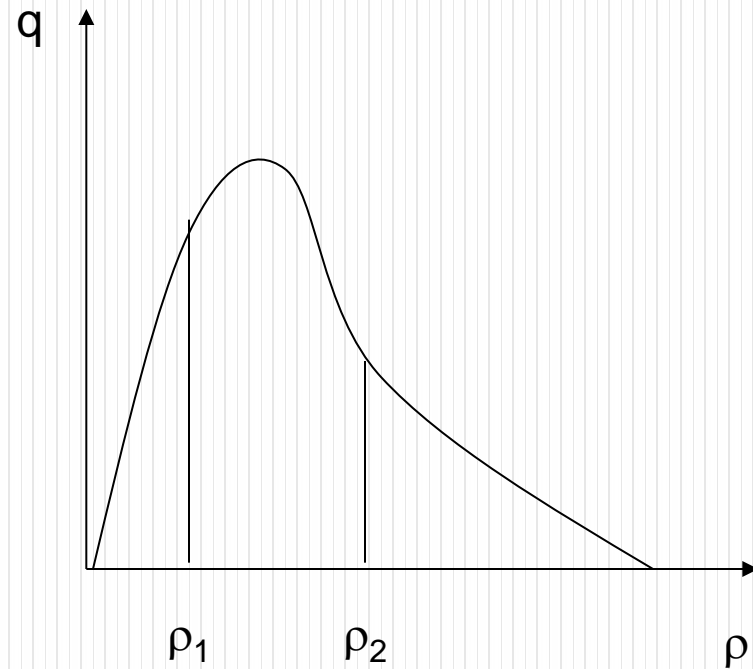
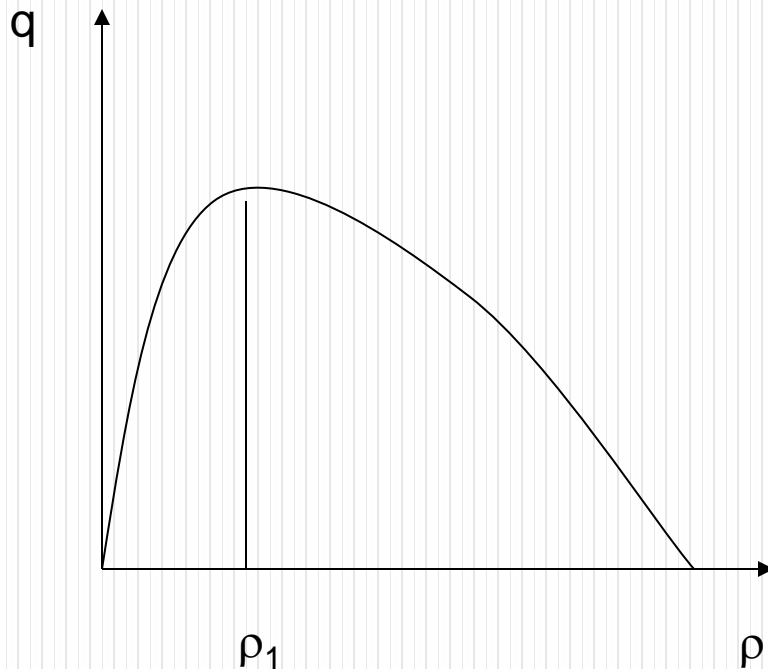
$$-\bar{\rho}^2 v'_*(\bar{\rho}) = u'_*(\bar{s})$$

$$u'_*(\bar{s}) > \frac{1}{2\tau}$$

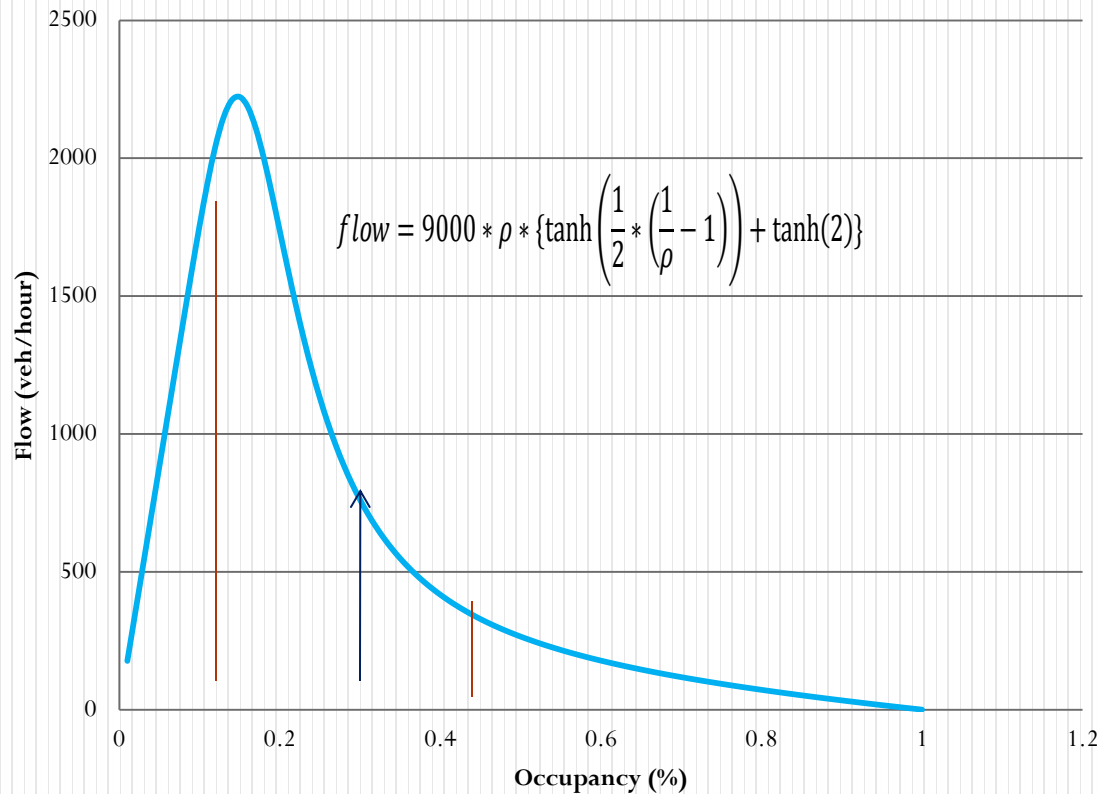
(Payne, Newell)

$$u'_*(\bar{s}) > \frac{a}{2}$$

(Bando)



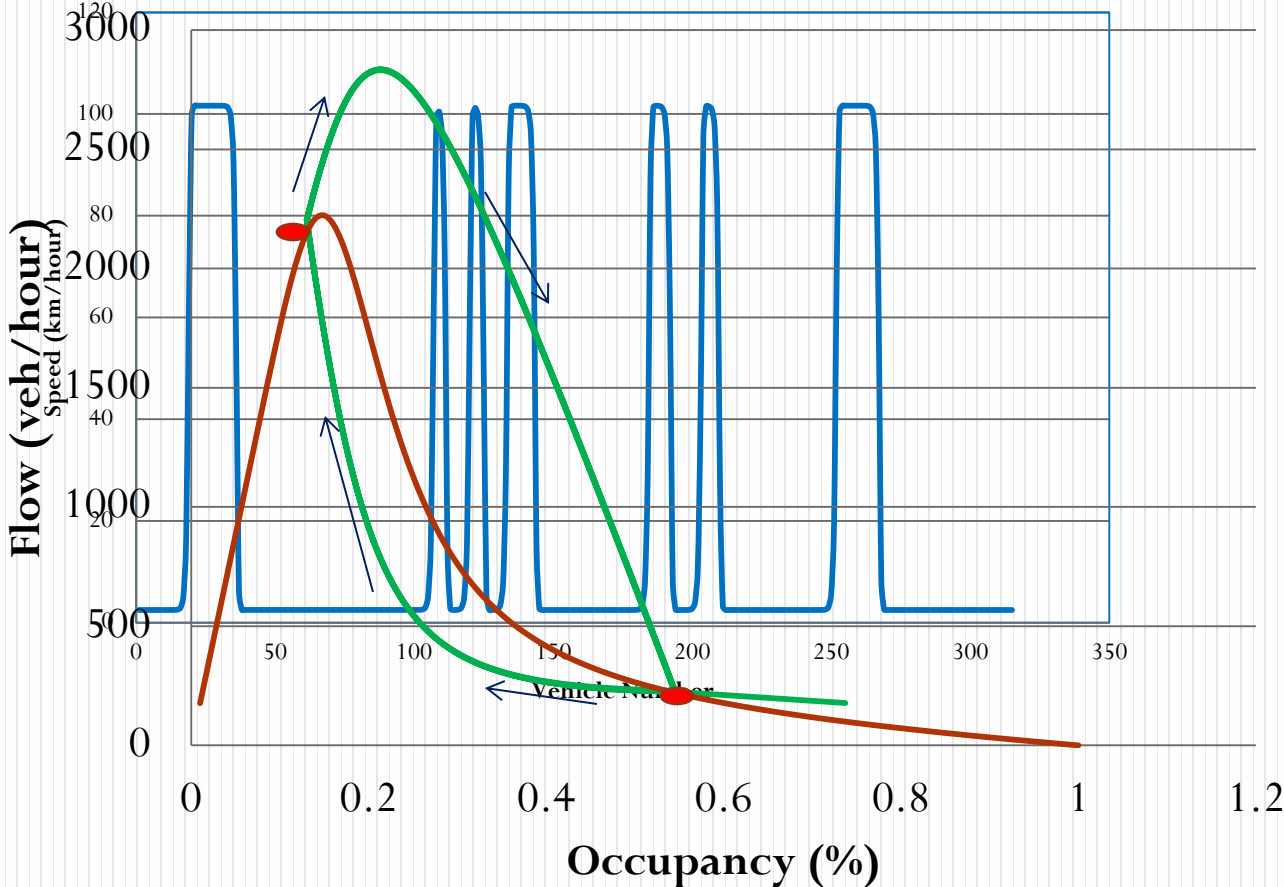
# Illustration: cluster solutions in the Bando model with a non-concave FD



$L=6,000$  m,  $l=6$ m,  $T=600$ s,  $dt=0.1$ s,  $\rho_j=167$  veh/km,  
 $N=300$  veh, average gap=14 m, Avg. occ is 0.3 .  
Vehicles randomly placed on circular road with 0 speed

# Illustration: cluster solutions in the Bando model with a non-concave FD

Snap shot at  $t=600s$





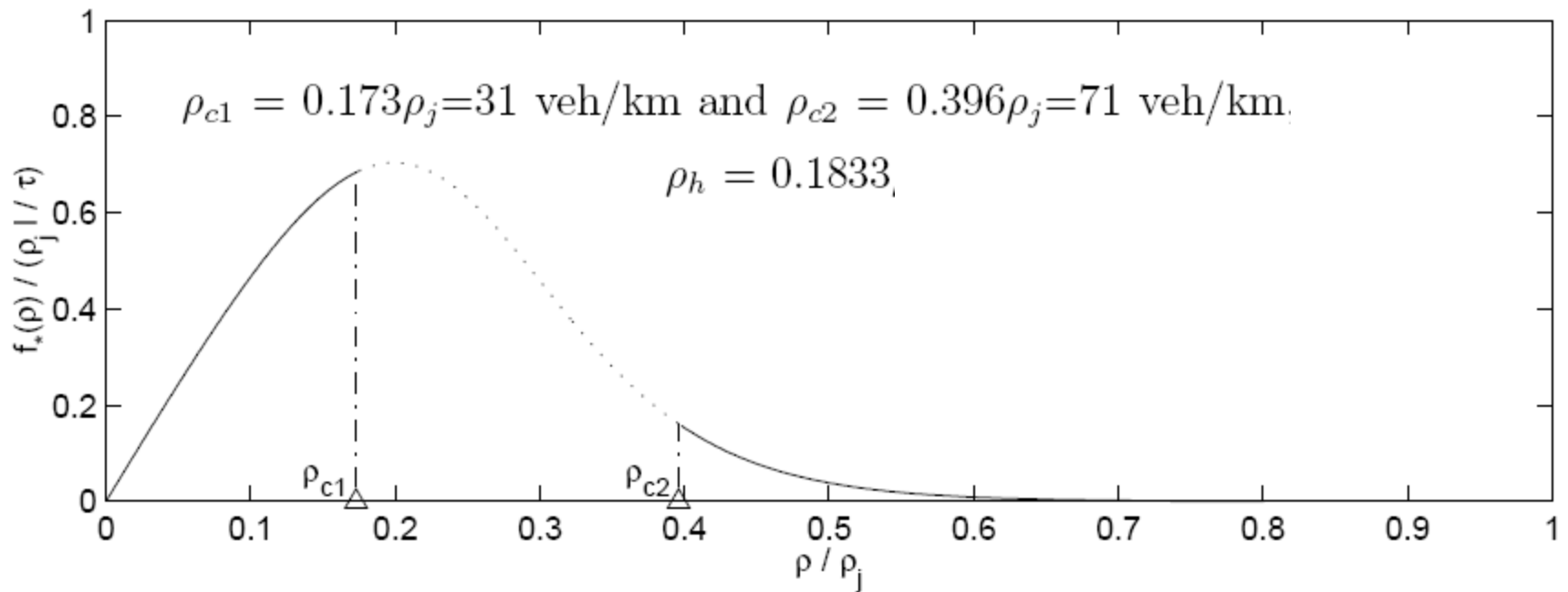
# Illustration: cluster solutions in PW model with a non-concave FD

$$\rho(x, 0) = \rho_h + \Delta\rho_0 \cos(2\pi x/L), \quad x \in [0, L],$$

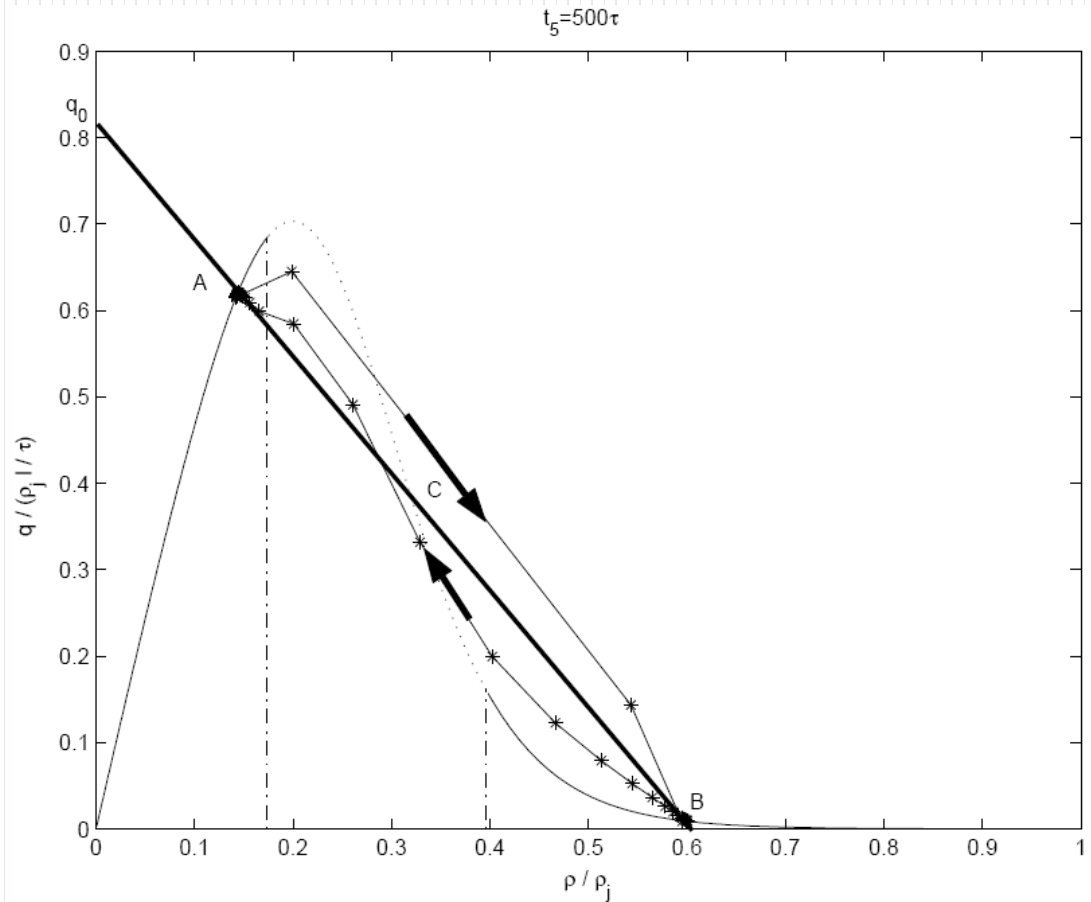
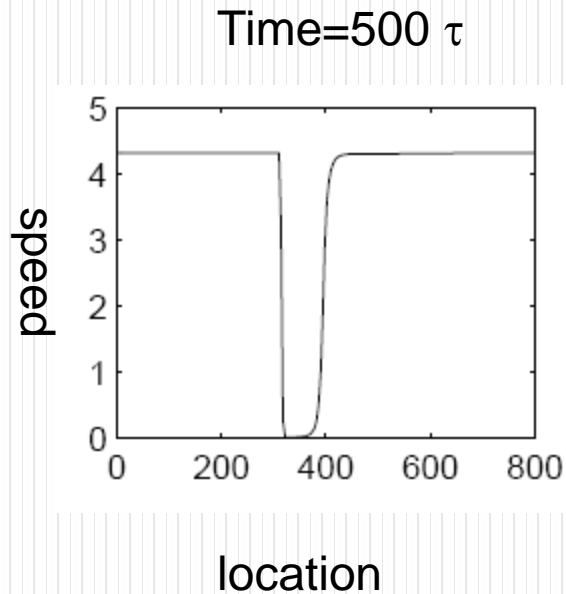
$$v(x, 0) = v_*(\rho_h) + \Delta v_0 \cos(2\pi x/L), \quad x \in [0, L];$$

$$L=22.4\text{km}, T=0.7\text{ hr}$$

$$\tau=5\text{s},$$



# Illustration: cluster solutions in PW model with a non-concave FD



# Concluding remarks

- The FD plays a pivotal role in traffic flow models and applications
- Diverse forms have been suggested in response to observed complexity in traffic flow
- Showed that the single-valued, continuous, non-concave FD, exhibits complexity with simplicity, and can be adopted to model quasi steady state flow (transitions between equilibrium states)

# Concluding remarks

- The FD plays a pivotal role in traffic flow models and applications
- Diverse forms have been suggested in response to observed complexity in traffic flow
- Provided a set of criteria for assessing FD forms
- Showed that the single-valued, continuous, non-concave FD, exhibits complexity with simplicity, and can be adopted to model quasi steady state flow (transitions between equilibrium states)

# Concluding remarks-continued

- One can extend the modeling capabilities of the simpler FDs by
  - ascribing each lane a FD to reflect lane differences in macro models
  - ascribing each group of drivers/vehicles a unique FD to reflect diversity in driver/vehicle units in micro models
- It is possible to incorporate more complex FDs, such as FDs with hysteresis, into micro or macro models, or in the extreme ascribe to each vehicle a unique complex FD.
- Realism, tractability, numerics

# Discovery of a New Hysteresis

